## SOLUTIONS MANUAL

# Wastewater Engineering:

Treatment and Resource Recovery Fifth Edition

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## 1

# INTRODUCTION TO WASTEWATER TREATMENT

#### PROBLEM 1-1

**Instructors Note:** The first six problems are designed to illustrate the application of the mass balance principle using examples from hydraulics with which the students should be familiar.

### Problem Statement - See text, page 53

#### Solution

Write a materials balance on the water in the tank
 Accumulation = inflow – outflow + generation

$$\frac{dV}{dt} = \frac{dh}{dt}A = Q_{in} - Q_{out} + 0$$

2. Substitute given values for variable items and solve for h

$$\frac{dh}{dt}A = 0.2 \text{ m}^3/\text{s} - 0.2 \left(1 - \cos\frac{\pi t}{43,200}\right) \text{ m}^3/\text{s}$$

$$A = 1000 \text{ m}^2$$

3. 
$$dh = 2 \times 10^{-4} \left( \cos \frac{\pi t}{43,200} \right) dt$$

Integrating the above expression yields:

$$h - h_o = \left[\frac{(43,200)(2x10^{-4})}{\pi}\right] \left(\sin\frac{\pi t}{43,200}\right)$$

4. Determine h as a function of time for a 24 hour cycle

Mass loading to plant, kg/h = 
$$[(V_T)(X_{ic}) + (V_{sp})(X_{sp})]/10^3$$
 g/kg)

For example, for the time period M-1, the mass loading rate is

= 
$$[(990 \text{ m}^3)(150 \text{ g/m}^3) + (116 \text{ m}^3)(214 \text{ g/m}^3)]/10^3 \text{ g/kg})$$

- = 174 kg/h (rounded)
- e. Set up a spreadsheet and computation table similar to that below.

1. Using the data given above, determine the tank dimensions.

V/Q = 3 min at peak flowrate (20,000 m<sup>3</sup>/d per channel)

$$V = \frac{(3\min)(20,000\,\text{m}^3\,/\,\text{d})}{(60\,\text{min/h})(24\,\text{h}\,/\,\text{d})} = 41.7\,\text{m}^3$$

$$A_s = \frac{41.7 \, \text{m}^3}{4 \, \text{m}} = 10.4 \, \text{m}^2$$

Length x width = 
$$4w \times w = 4w^2 = 10.4 \text{ m}^2$$

Width = 
$$1.6 \text{ m}$$

Length = 
$$6.4 \text{ m}$$

2. Determine the maximum air requirement.

$$Q_{air} = \left(\frac{0.3 \,\mathrm{m}^3}{\mathrm{m} \cdot \mathrm{min}}\right) \,(6.4 \,\mathrm{m})(2 \,\mathrm{channels}) = 3.84 \,\mathrm{m}^3/\mathrm{min}$$

3. Determine the horsepower requirements. Assume the blower efficiency is 70%. The specific weight of water is 9.81 kN/m³ (9,810 N/m³) (Appendix C). 1 W = 1 J/s or 1 N•m/s (Table 2 in Front section) Thus, 1 kW = 1000 N•m/s.

$$h = 0.250 \text{ m} \text{ (diffusers)} + 3 \text{ m (submergence)} + 0.40 \text{ m} = 3.65 \text{ m}$$

$$h = (3.65 \text{ m})(9810 \text{ N/m}^3) = 35,806 \text{ N/m}^2$$

$$Power = \left(\frac{Q_{air}h}{efficiency}\right)$$

4. Determine the power cost. Assume the electric motor efficiency is 92%.

Power cost = 
$$\left[\frac{(3.724 \text{ kW})(24 \text{ h/d})}{(0.9)}\right] (\$0.12 / \text{kWh}) = \$10.48 / \text{d}$$

Note: In the above computation, it was assumed that the blower operates at maximum capacity regardless of average flow or peak flow conditions. In small plants, this situation is often the case. Under actual operating

#### PROBLEM 7-3

**Problem Statement** – see text, page 675

**Instructors Note:** The purpose of this problem is to have the students familiarize themselves with appropriate literature for developing responses to the problem statement.

#### PROBLEM 7-4

**Problem Statement** – see text, page 675

**Instructors Note:** The purpose of this problem is to have the students familiarize themselves with appropriate literature for developing responses to the problem statement.

#### **PROBLEM 7-5**

**Problem Statement** – see text, page 675

#### Solution

- Prepare a COD balance to determine the amount of casein COD oxidized (O<sub>2</sub> consumed)
  - g COD cells + g COD oxidized = g COD removed
- 2. Determine COD of 22 g casein removed
  - a. Basic equation for casein oxidation:

$$C_8H_{12}O_3N_2 + O_2 \rightarrow CO_2 + H_2O + NH_3$$

b. Balance equation

$$\mathsf{C_8H_{12}O_3N_2} + 8.0\ \mathsf{O_2} \rightarrow 8\ \mathsf{CO_2} + 3\ \mathsf{H_2O} + 2\ \mathsf{NH_3}$$

- $8.0 \text{ moles } O_2 \text{ / mole casein}$
- c. Compute g COD removed

MW casein: 
$$8(12) + 12(1) + 3(16) + 2(14) = 184$$

g COD/g casein = 
$$\frac{8.0(32)}{184}$$
 = 1.39

g COD removed = (1.39 g COD/g casein)(22g) = 30.6 g

Chapter 8 Suspended Growth Biological Treatment Processes

$$S = \frac{8(1+0.1SRT)}{4.2SRT-1}g/m^3$$

At SRT = 
$$3.0 \text{ d}$$
, S =  $0.90 \text{ g/m}^3$ 

S as a function of SRT is summarized in table below.

b. Determine effluent  $NH_4$ -N ( $N_e$ ) concentrations as a function of SRT by combining Eq. (7-94) and Eq. (7-98) in Table 10. Let  $S_o = DO$ .

$$\frac{1}{SRT} = \left(\frac{\mu_{max,AOB}S_{NH4}}{K_{NH4} + S_{NH4}}\right) \left(\frac{DO}{K_o + DO}\right) - b_{AOB}$$

Solving for  $S_{NH4}$  (let  $S_{NH4} = N_e$ ):

$$N_{e} = \frac{K_{NH4}(1 + b_{AOB}SRT)}{[\mu_{max,AOB}\left(\frac{DO}{K_{o} + DO}\right) - b_{AOB}]SRT - 1}$$

$$N_{e} = \frac{0.50 \text{ g/m}^{3} [1 + (0.147 \text{ g/g-d}) \text{SRT}]}{\text{SRT} \left\{ 0.636 \text{ g/g-d} \left[ \frac{(2.0 \text{ g/m}^{3})}{(0.50 \text{ g/m}^{3} + 2.0 \text{ g/m}^{3})} \right] - 0.147 \text{ g/g-d} \right\}_{-1}}$$

At SRT = 3.0 days, 
$$N_e = 8.62g/m^3$$

Table showing Effluent  $NH_4$ -N ( $N_e$ ) and effluent sbCOD (S) concentration as a function of SRT:

SRT	Effl. sbCOD	Effl. NH <sub>4</sub> -N	SRT	Effl. sbCOD	Effl. NH <sub>4</sub> -N
d	mg/L	mg/L	d	mg/L	mg/L
3	0.90	8.62	12	0.36	0.42
4	0.71	1.79	13	0.34	0.39
5	0.60	1.08	14	0.33	0.38
6	0.53	0.81	15	0.32	0.36
7	0.48	0.66	16	0.31	0.35
8	0.44	0.58	17	0.31	0.34
9	0.41	0.52	18	0.75	0.24
10	0.39	0.47	19	0.73	0.23
11	0.37	0.44	20	0.72	0.22

3. The trickling filter clarifier area is the same as in the BOD removal only application as shown in Step A3 above.

Area =  $860 \text{ m}^2$ 

BOD removal only and BOD and nitrification design information is found in Example 8-3. These results are included in the summary comparison tables below.

### Summary Table (BOD removal only)

Parameter	Unit	Trickling filter	Activated sludge	
Aeration volume	$m^3$		4446	
Aeration depth	m		5.0	
Media volume	$m^3$	4672		
Media deph	m	6.1		
Aerobic tank area	$m^2$		889	
Trickling filter area	$m^2$	766		
Clarifier area	$m^2$	860	946	
Total area	$m^2$	1626	1835	
Pumping rate	m³/d	34,050		
Pumping energy	kW/10 <sup>3</sup> m <sup>3</sup> /d	1.5		
Air supply rate	m³/min		60.5	
Aeration energy	kW/m³•min		1.8	
Monthly energy	kWh/mo	38,736	78,408	

Note: Activated sludge monthly energy = (60.5)(1.8)(24)(30) = 78,408 kWh.

#### Summary Table (combined BOD and nitrification)

Parameter	Unit	Trickling filter	Activated sludge	
Aeration volume	$m^3$		13,418	
Aeration depth	m		5.0	
Media volume	$m^3$	25,608		
Media deph	m	6.1		
Aerobic tank area	$m^2$		2684	
Trickling filter area	$m^2$	4198		
Clarifier area	$m^2$	860	946	
Total area	$m^2$	5058	3630	
Pumping rate	m³/d	179,330		
Pumping energy	$kW/10^3  m^3/d$	1.5		
Air supply rate	m³/min		115.5	

2. The recovery rate is found with Eq. (11-40).

$$r,~\%~=~\frac{Q_p}{Q_f}\cdot 100 = \frac{(3650~m^3~/~d)}{(4000~m^3~/~d)}~x~100 = 91\%$$

3. Estimate the rejection rate using Eq. (11-41).

$$R,\% = \frac{C_f - C_p}{C_f} \cdot 100$$

$$R = \frac{[(191 - 65) g/m^3]}{(191 kg/m^3)} \times 100 = 66\%$$

4. Summary of results for Problem 11-17

		Reverse osmosis unit			
Item	Unit	1	2	3	4
C <sub>f</sub>	g/m <sup>3</sup>	191	329	1742	2583
$Q_p$	$m^3/d$	3650	5400	500	1000
r	%	91	90	6	10
R	%	66	73	93	93

#### PROBLEM 11-18

**Problem Statement** - See text, page 1283

#### **Solution**

1. Determine the permeate flowrate for water 1 using Eq. (11-40).

$$Q_p = \frac{r \cdot Q_f}{100} = \frac{(88\%)(4000 \text{ m}^3 / \text{d})}{100} = 3520 \text{ m}^3 / \text{d}$$

2. Rearrange Eq. (11-38) to compute the water mass transfer coefficient,  $k_{\rm W}$ .

$$k_w = \frac{(Q_p)}{(A)(\Delta P_a - \Delta \Pi)} = \frac{(3520 \text{ m}^3 / \text{d})(10^3 \text{kg/m}^3)}{(1600 \text{ m}^2)(2.7 \text{x} 10^6 \text{ kg/m} \cdot \text{s}^2)(86,400 \text{ s}/1\text{d})}$$
$$= 9.09 \text{ x} 10^{-9} \text{ s/m}$$

3. Estimate the concentrate stream TDS using Eq. (11-44).

$$C_c = \frac{Q_f C_f \cdot Q_p C_p}{Q_c}$$

 Determine the BOD influent concentration (BOD<sub>C</sub>) to the aeration tank. (Note: the flowrate of the primary clarifier underflow is neglected).

BOD<sub>C</sub> = 
$$\frac{(12,301 \text{ kg/d})(10^3 \text{ g/kg})}{(54,000 \text{ m}^3/\text{d})}$$
 = 228 mg/L

ii. Determine the mass of VSS produced that must be wasted using Eq. (8-19).

$$P_{x,VSS} = \frac{Y_{obs} Q(S_o - S)}{(10^3 g/kg)}$$

$$P_{x,VSS} = \frac{(0.3125^{*})(54,000 \text{ m}^{3} / \text{d})[(228 - 6.2^{*}) \text{ g} / \text{m}^{3}]}{(10^{3} \text{ g} / 1 \text{ kg})} = 3743 \text{ kg} / \text{d}$$

iii. Determine the TSS<sub>M</sub> that must be wasted.

$$TSS_M = (3743 \text{ kg/d})/0.80^* = 4679 \text{ kg/d}$$

iv. Determine the effluent mass quantities.

$$BOD_M = (54,000 \text{ m}^3/\text{d})(20 \text{ g/m}^{3*})/(10^3 \text{ g/kg}) = 1080 \text{ kg/d}$$
  
 $TSS_M = (54,000 \text{ m}^3/\text{d})(22 \text{ g/m}^{3*})/(10^3 \text{ g/kg}) = 1188 \text{ kg/d}$ 

v. Determine the waste quantities discharged to the thickener (assume wasting from the aeration tank).

$$TSS_M = (4679 - 1188) \text{ kg/d} = 3491 \text{ kg/d}$$

Flowrate = 
$$\frac{(3491 \text{kg}/\text{d})(10^3 \text{g}/\text{kg})}{(4375 \text{g}/\text{m}^3)^*}$$
 =  $798 \text{m}^3/\text{d}$ 

- d. Flotation thickener.
  - i. Determine the flowrate of the thickened sludge.

Flowrate = 
$$\frac{(3491 \text{ kg/d})(0.9^*)}{(10^3 \text{ kg/m}^3)(0.04)^*} = 78.5 \text{ m}^3 / \text{d}$$

ii. Determine the flowrate recycled to the plant influent.